# Effect of substrate thermal properties on evaporating liquid hydrogen and ammonia spills

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# Abstract

To decarbonize the energy, transport and industrial sectors, liquid hydrogen and ammonia are likely to be more widely employed. During an accidental release, these cryogens quickly spread and evaporate, producing explosive ( $H_2$ ) or toxic ( $NH_3$ ) clouds. Assessing the risks associated with storage and transport therefore requires tools that can simulate these spill processes, accounting for both the spill source, geometry and substrate thermal properties. In this work we have developed a flexible tool that takes the details of the spill, geometry and substrate as input. The parameters include initial spill velocity, ground topography, obstructions, and details regarding the thermal properties of the substrate. The latter includes temperature-dependent thermal properties, porosity and potential freeze out of trapped water. We validate this model against experimental data and apply it to relevant  $H_2$  and  $NH_3$  spill cases. Evaporation rates were found to vary significantly with substrate characteristics, and this is expected to have a large impact on safety distances.

Keywords: liquid hydrogen, liquid ammonia, spill modelling, safety, evaporation

# 1. Introduction

Hydrogen (H<sub>2</sub>) and ammonia (NH<sub>3</sub>) are expected to play an important role in the decarbonization of hard-to-abate sectors (van der Spek *et al.*, 2022). One of the challenges is their low energy density, which is mainly attributed to H<sub>2</sub> and NH<sub>3</sub> being in a gaseous state at standard conditions. The low energy density can be improved either by pressurization, or through a liquefaction process that takes the fluids to their liquid states at atmospheric pressure (-252.9 °C for H<sub>2</sub> and -33.34 °C for NH<sub>3</sub>). For large-scale storage and long-distance seaborne transport, liquefaction is a main option (Berstad *et al.*, 2022).

There are risks related to the storage, transport and end use of all fuels. One must therefore be able to assess these risks for each specific application. Before  $LH_2$  and  $LNH_3$  are employed in larger quantities and/or in new applications, a thorough understanding of the possible hazardous scenarios is required. One of the scenarios that must be considered is the accidental release of the liquids. Determining where the spilled liquid flows, and how it evaporates, is important, both for hazards directly related to the liquid (frost damage to personnel or ignition) and for assessing the consequences of vapour dispersion in the atmosphere (risk of ignition and, in the case of  $NH_3$ , toxicity).

Several experiments have been conducted where  $LH_2$  or  $LNH_3$  is spilled on land, potentially leading to the formation of an evaporating pool. The primary objective of these experiments has been to understand the dispersion of  $H_2$  (Witcofski and Chirivella, 1984; Statharas *et al.*, 2000; Willoughby and Royle, 2014; Hall, 2014; Aaneby *et al.*, 2021) and  $NH_3$  (Dharmavaram *et al.*, 2023; McMasters and Fox, 2020) in gaseous form in the atmosphere.

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In the 1980s, researchers from NASA conducted a series of seven experiments at the White Sands test site in New Mexico (Witcofski and Chirivella, 1984). The average spill rate for five of these tests was between 9.5 and 15 kg/s, considered to be large-scale experiments. The remaining two tests had an average spill rate of 4.7 kg/s and 0.8 kg/s. During the experiments, LH<sub>2</sub> was discharged by a helium back-pressure of 7 bar into a spill pond where it formed an evaporating pool on compacted sand. Test 6, which involved a liquid spill rate of  $\dot{m}_{\ell} = 9.5$  kg/s for a duration of 38 s, led to the formation of a pool with a radius between 2 and 3 m. This pool evaporated quickly once the spill ceased.

In the 1990s, Battelle Ingenieurtechnik for Bundesanstalt für Materialforschung und Prüfung (BAM) performed experiments where LH<sub>2</sub> was released between two buildings at a rate of around 0.4 kg/s during 2 minutes (Statharas *et al.*, 2000). The experiments were instrumented with sensors to measure pool size, ground temperature and evaporation behaviour. Unfortunately, the release rate is uncertain, especially because LH<sub>2</sub> was supplied through a 75 m long transfer line causing the liquid to heat and evaporate inside the pipe. An average liquid release rate of  $\dot{m}_{\ell} = 0.37$  kg/s was estimated.

In 2009–2011, the UK Health and Safety Laboratories (HSL) performed a series of tests in which unignited or ignited  $LH_2$  was released at a rate of around 0.07 kg/s onto concrete (Willoughby and Royle, 2014; Hall, 2014). The purpose of the tests was to mimic a scenario where an  $LH_2$  hose line fails during a tanker refuelling process. Three different orientations of the spill were investigated. Horizontal releases at a height of 0.86 m did not cause any pool formation. However, horizontal releases at ground level and vertical releases 0.1 m above the ground did result in a pool being formed.

In 2019 and 2020, the Norwegian Defence Research Establishment (FFI) performed seven outdoor and eight indoor  $LH_2$  leakage tests, where liquid pool formation was part of the assessment (Aaneby *et al.*, 2021). Five of the seven outdoor tests were released vertically downwards 0.32 m above the ground with a release rate in the range 0.16–0.73 kg/s. The two other tests were released horizontally with a release rate of 0.83 kg/s. Only the vertical releases resulted in pool formation. The radius was limited to 0.5–1 m from the release source and the pool evaporated shortly after the releases were stopped. A new series of experiments involving the release of  $LH_2$  was scheduled for 2023 at Sandia National Laboratories (Mangala Gitushi *et al.*, 2023).

Various computational tools (van den Bosch and Weterings, 2005; Verfondern and Dienhart, 1997; Holborn *et al.*, 2020) have been used to estimate the pool size for cryogenics, ranging from simple correlations for the radius to solving the shallow-water equations yielding a more precise description of the liquid flow. For the tools solving the shallow-water equations, several assumptions have typically been made to predict the pool evaporation rate. Firstly, the pools have been assumed to be in perfect thermal contact with the substrate. Moreover, the change in thermal properties as the substrate cools has not been accounted for. The latter may have a pronounced effect for cryogenic liquids such as LH<sub>2</sub>, for which the ground is significantly cooled.

Most of the experiments available in the literature do not provide accurate data on pool sizes and evaporation rates. This makes it challenging to adequately validate computational tools designed to estimate these parameters. Nonetheless, efforts have been made in this direction. Assuming axisymmetric spills, Verfondern and Dienhart (1997) used the LachenAusbreitung-und-Verdampfung (LAuV) code, developed by Forschungszentrum Jülich (FZJ), to simulate NASA Test 6 (Witcofski and Chirivella, 1984). They predicted a maximum pool radius around twice as large as the experimentally reported one. The FLACS pool model is another tool that solves the shallow-water equations in order to predict the evaporation of a liquid pool. It was employed by Holborn *et al.* (2020) to predict the pool radius for several spill rates, including that in NASA Test 6. Relatively good agreement with the experimentally reported radius was observed. The model assumptions included perfect thermal contact between the liquid and substrate, and constant thermal properties of the substrate, despite the cooling of the ground. The same study indicated that changing substrate, i.e., changing thermal properties, can significantly influence the pool radius and evaporation rate. Thus, it can be anticipated that the cooling of the ground, causing a transient change in ground thermal properties, may have a similar significant impact.

In this work, we therefore solve the shallow-water equations governing the flow of the liquid while continuously accounting for the change in thermal properties of the ground as it is cooled by the



Figure 1: Sketch of evaporation model.

cryogenic liquid. This allows us to determine if the assumption of constant thermal properties is adequate. Furthermore, we investigate the validity of the assumption of perfect thermal contact between the liquid and the substrate. This is particularly interesting to examine for both  $LH_2$  and  $LNH_3$ , since the two liquids typically have different boiling characteristics. The tool that has been developed, building on an open-source code, is freely available on Github (Grønli *et al.*, 2024). It is capable of simulating time-dependent spills of any shape with a nonzero initial velocity. Additionally, obstacles of varying shape and height may be included.

The rest of this article is organized as follows. Section 2 introduces the methodology to predict the spreading and continuous evaporation of  $LH_2$  and  $LNH_3$ . Section 3 discusses the effect of variable ground thermal properties, ground moisture, and heat transfer assumptions. Also, a geometrically more complex case showing the versatility of the model is considered. Section 4 provides concluding remarks.

# 2. Method

In the following, we present a method that predicts the spreading and continuous evaporation of LH<sub>2</sub> or LNH<sub>3</sub> on solid ground subsequent to a containment breach, see Fig. 1. Generally, a cryogenic liquid spill will spread across the ground surface while being heated by both the substrate and the atmosphere, leading to evaporation into the atmosphere. The approach is divided into two consecutive steps: (1) Solve the 1D heat equation to obtain the heat flux from the ground into the liquid,  $\dot{q}_g = \dot{q}_g(t_w)$ , where  $t_w(x, y)$  is the time a ground grid cell has been wet; (2) Solve the shallow-water equations to calculate the spreading and evaporation of the liquid.

In the second step, the ground heat flux obtained from the first step is utilized as input to determine the local evaporation rate. This one-way coupling approach disregards horizontal temperature gradients in the ground, yielding a heat flux solely dependent on the duration the ground grid cells have been covered by cold liquid,  $t_w$ . This should be a good approximation, as thermal gradients are in the range of  $10 \text{ K cm}^{-1}$  to  $100 \text{ K cm}^{-1}$  (e.g., shown in Fig. 3), while horizontal thermal gradients are determined by length scales of the spill (several meters). The computational cost saved by this exploitation of the underlying physics, and avoiding a 3D heat conduction problem with variable thermal properties, reduced the problem from completely unfeasible to a single 1D-simulation pre-processing calculation. This pre-processing step, depending on the complexity of the thermal properties, is done in the range of minutes to an hour. The following spill simulation time is some minutes. This also means that doing variations of geometry and spill source is fast if the substrate stays the same.

In case of an accidental release of LH<sub>2</sub> or LNH<sub>3</sub>, the size and local evaporation rate predicted here are important input parameters to dispersion analysis of NH<sub>3</sub> and H<sub>2</sub> in the atmosphere (see e.g., Skarsvåg

*et al.* (2024)). Allowing for temperature-dependent ground properties facilitates a detailed examination of the types of substrates that yield the least hazardous pool size and evaporation rate for a given spill scenario.

Before presenting the results for different thermal properties and substrates such as dry or wet sand, and dry or wet concrete, we describe the details of the two-step model.

#### 2.1. Pool spreading

In this work we have developed a new tool (Grønli *et al.*, 2024) building on the Conservation Laws Package (Clawpack) (Clawpack Development Team, 2022) to simulate the spill and spreading of  $LH_2$  and  $LNH_3$  on solid ground. Spreading of the liquid is modelled by solving the shallow-water equations on a two-dimensional Cartesian grid. The shallow-water equations are a set of hyperbolic partial differential equations derived from depth-integrating the Navier-Stokes equations, assuming that the horizontal length scale is much larger than the vertical length scale.

The first equation, derived from conservation of mass, reads

$$(h)_t + (hu)_x + (hv)_y = \frac{\dot{m}_{\ell} - \dot{m}_{\text{evap}}}{\rho_{\ell}},\tag{1}$$

in which *h* is the spill height (m), *u*, *v* are the velocities (m/s) in the *x*- and *y*-direction,  $\dot{m}_{\ell}$ ,  $\dot{m}_{\text{evap}}$  are the liquid mass spill and evaporation rates per unit area (kg/m<sup>2</sup> s), and  $\rho_{\ell}$  is the liquid density (kg/m<sup>3</sup>). Given a liquid-mass spill rate  $\dot{m}_{\ell}$ , the height at points in space where the liquid is spilled will increase at a rate (m/s)

$$\dot{h}_{\ell}(x,y,t) = \frac{\dot{m}_{\ell}(x,y,t)}{\rho_{\ell}}.$$
(2)

Similarly, at a given evaporation rate  $\dot{m}_{\rm evap}$ , the height will decrease at a rate

$$\dot{h}_{\text{evap}}(x, y, t) = \frac{\dot{m}_{\text{evap}}(x, y, t)}{\rho_{\ell}}.$$
(3)

The two remaining shallow-water equations, derived from conservation of momentum, are

$$(hu)_{t} + (hu^{2} + \frac{1}{2}gh^{2})_{x} + (huv)_{y} = -\gamma(hu) - ghb_{x},$$
(4)

$$(hv)_{t} + (hv^{2} + \frac{1}{2}gh^{2})_{y} + (huv)_{x} = -\gamma(hv) - ghb_{y},$$
(5)

where g is the gravitational acceleration (m/s<sup>2</sup>). The first source term on the right-hand side of both equations is a Manning friction term that includes the effect of substrate friction. The variable  $\gamma$  is given by

$$\gamma = \frac{gn^2 \sqrt{(hu)^2 + (hv)^2}}{h^{7/3}},\tag{6}$$

where the Manning coefficient n (s/m<sup>1/3</sup>) is correlated with ground surface roughness (The Engineering ToolBox, 2004). Next, the last term on the right-hand side of Eqs. (4) and (5) defines the topography where a nonzero elevation of the ground, b (m), is allowed. This is used to introduce obstacles or confinements of arbitrary shapes, such as dikes.

The shallow-water equations (1), (4) and (5) are solved by Clawpack (Clawpack Development Team, 2022) which is a collection of finite-volume methods for linear and nonlinear hyperbolic systems of conservation laws written in Fortran and Python. More specifically, we use the Python interface PyClaw together with a SharpClaw solver that provides high-order wave propagation using WENO (weighted essentially non-oscillatory) or TVD (total variation diminishing) reconstruction and Runge-Kutta time integration (Ketcheson *et al.*, 2013). The SharpClaw solver is selected as it tackles dry states where h = 0.

In this work we have modified PyClaw such that spills can be instantaneous (dam-break) or continuous, having a time-dependent spill rate. Various spill sizes and shapes, along with the option to assign a nonzero horizontal velocity, are also possible. For instance, for a spill rate  $\dot{m}_{\ell}$  with an initial velocity  $u_0$  in the *x*-direction, the resulting velocity in the *x*-direction within the spill domain becomes

$$u' = \frac{u_0 \dot{m}_{\ell} \Delta t + u m_{\ell}}{\dot{m}_{\ell} \Delta t + m_{\ell}},\tag{7}$$

where  $m_{\ell}$  denotes the mass of liquid already present inside the spill domain and  $\Delta t$  is the time step used in PyClaw.

#### 2.2. Heat conduction and evaporation

The liquid mass evaporation rate in shallow-water equation (1) can be estimated through the relation

$$\dot{m}_{\rm evap} = \frac{\dot{q}}{\Delta H_{\rm evap,\ell}},$$
(8)

in which  $\dot{q}$  is the heat flux into the pool per unit area (W/m<sup>2</sup>) and  $\Delta H_{\text{evap},\ell}$  is the liquid heat of evaporation at atmospheric pressure (J/kg). The liquid is assumed to stay at a saturated liquid state until complete boil-off, i.e., the liquid temperature  $T_{\ell} = T_{\text{sat}}$ , where  $T_{\text{sat}}$  is the saturation temperature. Evaporation is caused by heating from the ground and air at rates  $\dot{q}_g$  and  $\dot{q}_a$ , respectively. Other heat fluxes such as radiation to and from the atmosphere are neglected as these are small (Verfondern and Dienhart, 1997), at least during the initial stages of evaporation. Thus, the total heat flux into the pool is  $\dot{q} = \dot{q}_g + \dot{q}_a$ .

The heat flux from the air is given by

$$\dot{q}_{\rm a} = k_{\rm a}(T_{\rm a} - T_{\ell}),\tag{9}$$

where  $T_a$  is the temperature of the atmosphere (K) and the heat transfer coefficient,  $k_a$  (W/m<sup>2</sup>K), is estimated by assuming turbulent flow over a flat plat such that

$$k_{a} = \begin{cases} \frac{Nu\lambda_{a}}{2r_{p}} = \frac{0.037Pr_{a}^{1/3}Re^{0.8}\lambda_{a}}{2r_{p}} & \text{if } r_{p} \ge 0.1 \text{ m} \\ 0 & \text{if } r_{p} < 0.1 \text{ m} \end{cases}$$
(10)

in which  $r_p$  is the radius of the liquid pool (m),  $Pr_a$  is the Prandtl number of air (-) and  $\lambda_a$  is the thermal conductivity of air (W/mK) (van den Bosch and Weterings, 2005). The Reynolds number is given by

$$\operatorname{Re} = \frac{\rho_{a} u_{a,10} \cdot 2r_{p}}{\eta_{a}},\tag{11}$$

for which  $\rho_a$  is the density of air (kg/m<sup>3</sup>),  $u_{a,10}$  is the wind speed at 10 m height (m/s) and  $\eta_a$  is the dynamic viscosity of air (kg/m s).

The dominant contribution to evaporation is the ground heat flux,  $\dot{q}_{\rm g}$ . We expect vertical gradients in ground temperature to be much larger than horizontal temperature gradients. Thus, we can disregard heat transport in the horizontal directions and employ a 1D heat-transfer model. Hence, the heat flux from the ground is solely determined by the duration for which the ground remains covered by liquid. The temperature profile in the ground is given by the 1D heat equation

$$\frac{\partial}{\partial z} \left( \lambda_{\rm g} \frac{\partial T_{\rm g}}{\partial z} \right) = \rho_{\rm g} c_{\rm p,g} \frac{\partial T_{\rm g}}{\partial t},\tag{12}$$

where  $\lambda_{g}$ ,  $\rho_{g}$  and  $c_{p,g}$  are the ground thermal conductivity, density and specific heat capacity (J/kgK), respectively.

A simple approach is to assume temperature-independent (constant) properties  $\lambda_{g}$ ,  $\rho_{g}$  and  $c_{p,g}$  along with perfect thermal contact between the ground and the liquid pool. The resulting heat equation (12) can be solved analytically and the ground heat flux is then given by

$$\dot{q}_{\rm g} = \frac{\lambda_{\rm g} \left( T_{\rm g}^{\infty} - T_{\ell} \right)}{\sqrt{\pi \, \alpha_{\rm g} t_{\rm W}}},\tag{13}$$

where  $\lambda_g$  and  $\alpha_g$  are the constant thermal conductivity and diffusivity (m<sup>2</sup>/s) of the ground. Moreover,  $T_g^{\infty}$  is the ground temperature at infinite depth and  $t_w$  is the time the ground has been wet.

However, as this is potentially too simplistic, we have tested the effect of accounting for temperaturedependent (variable) thermal properties  $\lambda_g$ ,  $\rho_g$  and  $c_{p,g}$ , along with the distinct boiling regimes experienced by LNH<sub>3</sub> and LH<sub>2</sub>. Hence, Eq. (12) is solved numerically utilizing the Python library pyPDE (Jackson, 2019). Among the substrates that have been considered are dry and wet saturated sand. The thermal conductivity of saturated wet soil can be approximated by

$$\lambda_{g,\text{sat}(u)} = \lambda_s^{1-n_u} \lambda_{w(u)}^{n_u},\tag{14}$$

where  $\lambda_s$  and  $\lambda_{w(u)}$  are the thermal conductivities of solid particles (quartz) and unfrozen water, respectively, while the porosity of the unfrozen soil is set to  $n_u = 0.335 \text{ m}^3/\text{m}^3$  (Côté and Konrad, 2005a). Due to the low temperatures of LNH<sub>3</sub> and LH<sub>2</sub>, soil containing water will freeze as it is cooled. The thermal conductivity of frozen saturated soil is calculated by

$$\lambda_{g,sat(f)} = \lambda_s^{1-n_f} \lambda_{w(f)}^{n_f}, \tag{15}$$

in which  $\lambda_{w(f)}$  is the thermal conductivity of frozen water and  $n_f$  is the porosity of frozen soil. Côté and Konrad (2005b) estimated a 9 % volume increase as pore water freezes to ice, implying a frozen porosity  $n_f = 1.09n_u/(1 + 0.09n_u)$ . The effect of the transition from unfrozen to frozen saturated soil on thermal conductivity has been modelled by

$$\lambda_{g,sat}(T) = \frac{1}{2} \left[ 1 + \tanh \beta (T - 271.15 \text{ K}) \right] \lambda_{g,sat(u)}(T) + \frac{1}{2} \left[ 1 - \tanh \beta (T - 271.15 \text{ K}) \right] \lambda_{g,sat(f)}(T),$$
(16)

where  $\beta$  is a parameter with unit 1/K controlling how fast the transition occurs. Here, we have assumed 2 K subcooling and neglected any significant freezing point depression. The same model has been applied to dry sand with air filling the pores instead of water, yielding a thermal conductivity  $\lambda_{g,dry} = \lambda_s^{1-n_u} \lambda_a^{n_u}$  where  $\lambda_a$  is the thermal conductivity of air. Herein, we have assumed that condensation of air components, which could occur for LH<sub>2</sub> spills, has a limited influence on the spill-evaporation dynamics. Numerical values for thermal conductivity have been obtained from Dincer and Zamfirescu (2015); Carnahan *et al.* (2021); Simoncelli *et al.* (2023); Kadoya *et al.* (1985); Dahmani *et al.* (2007) and are plotted in Fig. 2a for different substrates.

The specific heat capacity is assumed to equal the mass-weighted sum of the specific heat capacities for each constituent (quartz and water/air). For wet sand we must account for the freezing of ground moisture. The individual specific heat capacities of unfrozen (u) and frozen (f) wet sand read

$$c_{p(u)}(T) = w_{u}c_{p,w(u)}(T) + (1 - w_{u})c_{p,s}(T),$$
(17)

$$c_{p(f)}(T) = w_f c_{p,w(f)}(T) + (1 - w_f) c_{p,s}(T),$$
(18)

where  $c_{p,w(u)}$ ,  $c_{p,w(f)}$  and  $c_{p,s}$  are the specific heat capacities of unfrozen water, frozen water and quartz, respectively. The water content (% by weight) is  $w_u = S_r n_u \rho_{w(u)} / (\rho_s(1 - n_u))$  for the unfrozen state and  $w_f = S_r n_f \rho_{w(f)} / (\rho_s(1 - n_f))$  for the frozen state. Here,  $S_r$  is the degree of saturation,  $\rho_s$  is the density of quartz and  $\rho_{w(u)}$ ,  $\rho_{w(f)}$  are the densities of unfrozen and frozen water, respectively.



Figure 2: Thermal conductivity  $\lambda_g$  (a) and effective specific heat capacity  $c_p^{\text{eff}}$  (b) of different substrates.

With Eqs. (17) and (18), the specific heat capacity of wet saturated sand can be modelled in the same way as thermal conductivity in Eq. (16). However, due to freezing of ground moisture we add the heat of solidification of water,  $\Delta H_{sol,w}$ , through a sharp Gaussian distribution with standard deviation  $\sigma \approx 1/\beta$ . This yields an effective specific heat capacity of wet saturated sand

$$c_{p}^{\text{eff}} = \frac{1}{2} \left[ 1 + \tanh \beta (T - 271.15 \text{ K}) \right] c_{p(u)}(T) + \frac{1}{2} \left[ 1 - \tanh \beta (T - 271.15 \text{ K}) \right] c_{p(f)}(T) + \frac{w_{u} \Delta H_{\text{sol},w}}{\sigma \sqrt{2\pi}} \exp \left\{ -\frac{1}{2} \frac{(T - 271.15 \text{ K})^{2}}{\sigma^{2}} \right\}.$$
(19)

Note that if the substrate is rapidly cooled from above the freezing point to well below, the released energy shows little sensitivity to the magnitude of  $\sigma$ . Numerical values for specific heat capacity have been found in Lemmon *et al.* (2024, 2000); Harvey (2019) and are plotted in Fig. 2b, where the parameter  $\sigma$  has been set to 3.33.

In Figs. 2a and 2b, the thermal properties of concrete have also been plotted. These have not been calculated from the above equations but directly obtained from Dahmani *et al.* (2007). Here, the experimentally determined thermal properties of wet concrete are given for concrete that was first dried and then wetted (i.e., rewetted). These data were only available at temperatures relevant to  $LNH_3$  spills. To illustrate the effect of variable thermal properties for dry concrete, we extrapolate the constant values (dashed lines in Figs. 2a and 2b). In doing so, we assume the same temperature dependency as for dry sand, offset by the values at 10 °C for dry concrete. These extrapolated values are only used in Section 3.1.1.

It is observed that the ground density, whether in the unfrozen state,  $\rho_{g(u)}$ , or the frozen state,  $\rho_{g(f)}$ , exhibits minimal variation with changes in temperature. Hence, both are assigned a constant value that represents a weighted average of each component in the ground. However, for dry sand, the temperature dependence of air density,  $\rho_{a}$ , is accounted for by assuming ideal gas behaviour.

Assuming perfect thermal contact between ground surface and liquid as in Eq. (13) gives the maximal possible ground heat flux,  $\dot{q}_g$ . However, the heat flux will be limited by the boiling regime. This is governed by the temperature difference  $\Delta T_{g\ell} \equiv T_g^0 - T_\ell$ , where  $T_g^0$  is the ground surface temperature. For ground temperatures above the Leidenfrost temperature,  $T_L$ , the cryogen will film boil. The Leidenfrost temperature can be estimated by the expression  $T_L = 27T_c/32$ , where  $T_c$  is the critical temperature of the fluid (Spiegler *et al.*, 1963). We find  $T_L^{H_2} = 28$  K for  $H_2$  and  $T_L^{NH_3} = 342$  K for

 $NH_3$ . This means that initially  $H_2$  will always film boil, while  $NH_3$  will be in the nucleate boiling regime.

For  $LH_2$  boiling on flat horizontal surfaces it is recommended by Baldwin *et al.* (2021) to use Klimenko's film boiling correlation (Klimenko and Shelepen, 1982). Hence, the ground heat flux for boiling  $LH_2$  reads

$$\dot{q}_{g,H_2} = \begin{cases} 0.19\Delta T_{g\ell}\lambda_{\ell}B^{0.333} Pr_V^{0.333} f_1(K) / \lambda_{cr} & \text{if } B < 10^8 \\ 0.0086\Delta T_{g\ell}\lambda_{\ell}B^{0.5} Pr_V^{0.333} f_2(K) / \lambda_{cr} & \text{if } B > 10^8, \end{cases}$$
(20)

where  $\lambda_{\ell}$  is the thermal conductivity of LH<sub>2</sub> and Pr<sub>v</sub> is the Prandtl number of the vapour film formed between the ground surface and the liquid. Further, *B* is a variable given by

$$B = \frac{\lambda_{\rm cr}^3 g}{\nu_{\rm v}^2} \left( \frac{\rho_{\ell}}{\rho_{\rm v}} - 1 \right),\tag{21}$$

in which the critical (Laplace) length scale,  $\lambda_{cr}$  (m) is

$$\lambda_{\rm cr} = 2\pi \sqrt{\frac{\sigma_{\ell}}{g(\rho_{\ell} - \rho_{\rm v})}}.$$
(22)

Herein,  $\sigma_{\ell}$  is the surface tension (N/m),  $\rho_{v}$  is the vapour film density and  $v_{v}$  is the kinematic viscosity of the vapour film (m<sup>2</sup>/s). Moreover, in Eq. (20) we have

$$f_1(K) = \begin{cases} 1 & \text{if } K \le 1.4\\ 0.89K^{0.333} & \text{if } K > 1.4 \end{cases}$$
(23)

and

$$f_2(K) = \begin{cases} 1 & \text{if } K \le 2\\ 0.71K^{0.5} & \text{if } K > 2, \end{cases}$$
(24)

where

$$K = \frac{\Delta H_{\text{evap},\ell}}{c_{\text{p},\text{v}}\Delta T_{g\ell}}.$$
(25)

All vapour film properties (v), such as the specific heat capacity of the vapour film,  $c_{p,v}$ , are evaluated at the vapour film temperature estimated as  $T_v = (T_\ell + T_g^0)/2$  (Sciance, 1966). The vapour film acts as an insulator which considerably decreases the heat flux compared to a nucleate boiling regime. Liquid properties ( $\ell$ ) are evaluated at the liquid temperature  $T_\ell = T_{sat}$ .

For LNH<sub>3</sub> we impose Mostinski's nucleate boiling correlation (Mostinski, 1963)

$$\dot{q}_{\rm g,NH_3} = 1.167 \cdot 10^{-8} p_{\rm crit}^{2.3} F_{\rm PF}^{10/3} \Delta T_{\rm g\ell}^{10/3}$$
(26)

as a boundary condition to Eq. (12). Here,  $p_{crit}$  is the critical pressure of LNH<sub>3</sub> given in kPa. Moreover,  $F_{PF}$  is a non-dimensional pressure correction factor that characterizes pressure effects on nucleate boiling as

$$F_{\rm PF} = 1.8p_{\gamma}^{0.17} + 4p_{\gamma}^{1.2} + 10p_{\gamma}^{10}, \tag{27}$$

in which  $p_r = p/p_{crit}$  is the reduced ambient pressure (-) where we assume atmospheric pressure,  $p = p_a$  (Pa).

The heat flux calculated by Eq. (20) for  $LH_2$  or Eq. (26) for  $LNH_3$  will then serve as an input to shallow-water equation (1) via Eq. (8). The resulting temperature profiles for different substrates being covered by  $LH_2$  after 100 s are plotted in Fig. 3. Here we see the significant impact of moisture in the ground: dry sand has the lowest heat capacity and is thus cooled more easily, while wet sand can release much more heat without the same cooling, due to increased heat capacity and latent heat. Note that it is the surface temperature that determines the heat transfer to the  $LH_2$ . Due to decreased ground surface temperature, a possible transition to natural convection for  $LNH_3$  and transition boiling for  $LH_2$  can be expected for longer simulations than those we have run here.



Figure 3: Temperature profiles in sand covered by LH<sub>2</sub> after 100 s. "Wet sand,  $\Delta H_{sol,w} = 0$ " denotes that the heat of solidification is excluded. Variable thermal properties and boiling correlations are used with an initial ground temperature  $T_g(t = 0) = 10$  °C.

### 3. Results and discussion

In the following, we examine how the thermal properties of the substrate, as well as the contact between liquid and substrate affect liquid spreading and evaporation rate. Firstly, we validate our tool by comparing the ground temperature from our simulations with experimental data for  $LH_2$  provided by the Norwegian Defence Research Establishment (FFI) (Aaneby *et al.*, 2021). The temperature profile in the ground primarily determines the heat flux, which significantly influences the evaporation rate. Next, our tool is validated against experimental data from NASA Test 6 (Witcofski and Chirivella, 1984) and compared to FLACS pool model results (Holborn *et al.*, 2020). Subsequently, we conduct a case study to examine the effect of accounting for the temperature dependency of ground thermal properties. Moreover, we discuss the assumption of perfect thermal contact between the liquid and substrate by employing boiling correlations. Finally, the case study assesses the effect of moisture in the ground for sand and concrete.

The pool spreading is simulated using PyClaw on a 2D grid with a grid spacing and time step  $\Delta t$ , satisfying a maximal CFL (Courant-Friedrichs-Lewy) number of 0.9. The numerical parameters are summarized in Table 1, where "dry tolerance" refers to the maximal height *h* at which the substrate is considered to be dry. As the pool approaches complete boil-off and becomes increasingly shallow, the radius could be particularly sensitive to this parameter. In the same table, the grid spacing and time-step length for solving the heat equation (12) in Python are given.

Table 2 lists the main physical parameters. Herein, the Manning coefficient *n*, controlling the bottom friction, is set to 0.018 s/m<sup>1/3</sup> (The Engineering ToolBox, 2004). All spills are centred at (x, y) = (0, 0) with zero initial velocities  $u_0$ ,  $v_0$ , unless otherwise stated. For all cases in the case study, the initial ground temperature is  $T_g^0 = 10$  °C and the air temperature is  $T_a = 10$  °C. Moreover, the wind speed at 10 m height is  $u_{a,10} = 2$  m/s, while the liquid temperature is  $T_{\ell} = T_{sat}$ . Thermodynamic and transport properties of LH<sub>2</sub>, LNH<sub>3</sub> and water are obtained from the NIST Webbook (Lemmon *et al.*, 2024) or using Thermopack with NIST-MEOS as equation of state (Wilhelmsen *et al.*, 2017).

## 3.1. Validation against experiments

In this subsection, we validate our tool against the experiments conducted by FFI and NASA, described in Section 1, and compare it to FLACS pool model results.



Figure 4: Validation against FFI experiments. Temperature as a function of time in concrete covered by LH<sub>2</sub> at a depth of z = 20 mm and z = 30 mm in (a) and (b), respectively. Black curves represent experimentally recorded values at different radii r from the spill point and at different angles around it. Full lines are calculated with constant or variable properties, and perfect thermal contact (PTC) or film boiling (FB).

Table 1: Numerical parameters.

Variable	Value
Grid spacing for pool spreading	10 <sup>-2</sup> m
Max. CFL number for pool spreading	0.9
Dry tolerance for pool spreading	$10^{-5} {\rm m}$
Grid spacing for heat equation	$5 \cdot 10^{-5} \text{ m}$
Time step for heat equation	$10^{-5} { m s}$
Parameter in Eq. (19), $\sigma$	[3.33, 7.33] K

Variable	Value
Manning coefficient <i>n</i>	$0.018 \text{ s/m}^{1/3}$
Unfrozen porosity of sand $n_{\rm u}$	0.335
Degree of saturation for wet sand $S_r$	1
Initial ground temperature $T_{g}^{0}$ in Section 3.1.1	0°C
Initial ground temperature $T_{g}^{0}$ in Section 3.2	10 °C
Air temperature <i>T</i> <sub>a</sub>	10 °C
Wind speed at 10 meters height $u_{a,10}$	2 m/s
Density of quartz	2650 kg/m <sup>3</sup>
Density of concrete	2200 kg/m <sup>3</sup>

Table 2: Physical parameters.

# 3.1.1. Validation against FFI experiments

In the experiments conducted by FFI,  $LH_2$  was released onto concrete (Aaneby *et al.*, 2021). Thermocouples were used to measure the ground temperature at depths of 20 mm and 30 mm and radii of 0.2 m and 0.5 m from the spill source, at various angles around it. Figs. 4a and 4b display the ground temperatures calculated by our model alongside the experimentally reported temperatures for Test 7. Test 7 involved a vertical downward release with a mass outflow rate of 0.16 kg/s over 8 minutes and an  $LH_2$ -tanker pressure of 0.8 barg. As seen in Figs. 4a and 4b, the experimental values vary significantly with the angle around the spill source. The variations may be explained by the difficulty of measuring such low temperatures, the potential of inaccuracies in thermocouple placement at the specified depths and  $LH_2$  penetration into the ground, as suggested in Aaneby *et al.* (2021).

The experimental values are compared to our model, which is run with both constant and variable thermal properties, and under the assumption of perfect thermal contact or film boiling. Since thermal properties are unavailable for concrete at low temperatures, we assume that the thermal properties of dry concrete have the same temperature dependency as for dry sand, offset by the values at 10 °C for dry concrete, as shown in Figs. 2a and 2b. The ground temperatures calculated by our model all fall within the range of the experimentally measured temperatures. However, it can be seen that the scatter in the experimental data is in the order of the deviation between the data and the simulations. Overall, accounting for the variability in ground thermal properties gives better agreement with the experimental data than the film-boiling assumption. This could be explained by an under-prediction of the heat transfer by the film-boiling correlation, which could be due to the effects of the thermal properties, which can be seen by the significant difference between the constant thermal properties case and that of the variable properties in Fig. 4.

## 3.1.2. Validation against NASA experiments and comparison to FLACS simulations

In Fig. 5, the experimentally reported pool radius is indicated for NASA Test 6, having a spill rate  $\dot{m}_{\ell} = 9.5$  kg/s and duration of 38 s. The pool was observed to have a radius between 2 and 3 m, and to evaporate after 43.5 s (Holborn *et al.*, 2020). We simulate this release by employing the film boiling correlation for H<sub>2</sub> and variable ground properties for dry sand. Our model overestimates the radius and the time until complete boil-off. Since the upstream pressure exceeded atmospheric pressure, flashing is expected. However, this phenomenon was not considered in the calculations to ensure a conservative estimation of the maximum pool radius. Therefore, the discrepancy in the radius could be due to an actual liquid spill rate that was lower than reported. E.g., 41 % flashing was assumed by Verfondern and Dienhart (1997).

Once the liquid forms a pool, multiple reasons may cause overestimation of radius and boil-off time. The bottom friction modelled by the Manning coefficient n in Eqs. (4) and (5) may be larger than expected. However, the pool radius is not particularly sensitive to changes in n for such rapid spills. Next, underestimation of the heat flux could be a factor. However, as is later shown in Figs. 9a and 9b for LH<sub>2</sub>, even in the limit of perfect thermal contact, the behaviour remains similar. Larger thermal conductivity or moisture content could contribute to reducing the radius and boil-off time, but to get a significant change in radius, nonphysical values of the thermal-transport properties have to be used. In this NASA test, the trajectory of LH<sub>2</sub> was directed towards a deflection plate prior to reaching the ground. When 30% to 40% of the liquid mass flashes, the volume increases about 20 times. This would lead to a significant increase in the velocity. With a high-velocity impact, significant splashing is expected, which could be a large source of evaporation when droplets mix with the warm air. Detailed modelling of the impact zone including splashing, which our tool does not directly account for, would require a more sophisticated multiphase 3D CFD model. Additionally, measurement uncertainty, particularly of the spill radius, could play a role.

Holborn *et al.* (2020) utilized the FLACS pool model and found relatively good agreement with the radius reported in NASA Test 6. However, this was done with wet coarse sand as substrate, for which they selected a thermal conductivity of 3.72 W/m K and thermal diffusivity of  $1.45 \cdot 10^{-6} \text{ m}^2/\text{s}$ . For dry sand, which better replicates the experimental conditions, our model predicts a thermal conductivity of 0.94 W/m K and thermal diffusivity of  $4.88 \cdot 10^{-7} \text{ m}^2/\text{s}$  at 0 °C. Hence, the thermal conductivity used by Holborn *et al.* (2020) is about 400% higher than for dry sand.

To compare the results from FLACS with ours, we employ the same spill zone and constant thermal properties in the ground, along with the same simplification of perfect thermal contact. Moreover, following Holborn *et al.* (2020), we linearize the ground heat flux in Eq. (13) for  $t_w < 4$  s. The maximal pool radius differs from that obtained by FLACS by less than 5 %, but the time until complete boil-off is 10 % longer. A different definition of what is regarded as dry ground (see "dry tolerance" in Table 1) may contribute to explain the latter. Additionally, the larger grid spacing used by Holborn *et al.* (2020) may account for the minor discrepancies in radius. The smaller maximal radius assuming different constant thermal properties demonstrates that the estimate of thermal properties in the ground significantly affects the predicted size and evaporation rate of cryogenic pools. The high thermal conductivity used by Holborn *et al.* (2020) may serve to compensate for the effects of flashing and splashing, and explain the difference between the red and black lines in Fig. 5.

## 3.2. Effect of substrate variations and thermal-transport modelling choices

In the following subsections we investigate the effect changing the substrate, different treatment of thermal properties (temperature dependent vs. constant) and the effect of the solid-to-liquid heat-transfer model used. As a base case we use the NASA Test 6 (Witcofski and Chirivella, 1984) described in Sec. 1, and do variations on this case. The base case thus corresponds to the validation case used in Subsection 3.1.2, corresponding to the red line in Fig. 5. We use the same mass flow rate, for both  $H_2$  and  $NH_3$  spills, and consider both wet and dry sand as substrate. Subsequently, we consider the effect of moisture in concrete. The main cases are summarized in Table 3, in which Cases A–F are axisymmetric releases inside a spill zone having radius 0.75 m with duration 38 s and spill rate



Figure 5: The grey shaded area depicts the experimentally reported pool radius in NASA Test 6. The red line indicates the radius calculated by assuming variable thermal properties and film boiling (FB). The full black line was calculated by assuming perfect thermal contact (PTC) and constant thermal properties to be comparable to Holborn *et al.* (2020) (dotted line).



Figure 6: Sketch of an axisymmetric evaporating spill centred at (x, y) = (0, 0) with radius  $r_p$  where the spill height h(x, y, t) = h(r, t).

 $\dot{m}_{\ell}$  = 9.5 kg/s. This is illustrated in Fig. 6. Finally, we consider a more complex Case X to showcase the versatility of the tool.

# 3.2.1. Cases A and B: Constant vs. variable thermal substrate properties

The standard approach used in the literature to determine the pool size and evaporation rate for cryogenic liquids is to assume temperature-independent (constant) thermal properties in the ground (Holborn *et al.*, 2020; Verfondern and Dienhart, 1997). To check the validity of this assumption for spills of LH<sub>2</sub> and LNH<sub>3</sub> on dry sand, we have compared the pool radius and evaporation rate for spill Cases A and B. The constant substrate properties are set equal to the properties of dry sand at T = 10 °C. The boiling correlations in Eqs. (20) and (26) are employed for LH<sub>2</sub> and LNH<sub>3</sub>, respectively. As seen in Figs. 2a and 2b, there are large variations in thermal properties for the temperatures relevant for LH<sub>2</sub>. Thus one could expect a significant impact on the LH<sub>2</sub> case, and less so for the LNH<sub>3</sub> case. However, for dry sand, Figs. 7a and 7b show only insignificant differences in both radius and evaporation rate for constant and variable ground properties for LH<sub>2</sub>. We have verified that the same is also the case for LNH<sub>3</sub>.

We now switch the substrate to wet sand and extend the LH<sub>2</sub> spill duration to 900 s. The extended spill duration causes more cooling of the ground which increases the effect of variable thermal

Table 3: Spill cases with different substrates and their thermal properties. The right-most column indicates whether boiling correlations for film and nucleate boiling are employed or if perfect thermal contact is assumed.

Case	Substrate	Properties	Boiling
А	Dry sand	Variable	Film/Nucleate
В	Dry sand	Constant	Film/Nucleate
С	Dry sand	Variable	Perfect contact
D	Wet sand	Variable	Film/Nucleate
E	Dry concrete	Variable	Film/Nucleate
F	Wet concrete	Variable	Film/Nucleate
Х	Dry concrete	Variable	Nucleate
	and wet sand		

properties. Instead of selecting the thermal properties at the initial ground temperature,  $T_g^0 = 10$  °C, the constant thermal properties are set equal to the thermal properties of the ground at T = -1 °C. This choice is made since cooling to the freezing point is fast, and most of the heat conduction happens through soil at sub-zero temperatures. Evaluating the thermal properties at the initial temperature can lead to an underestimation of heat conduction (see Fig. 2a). The latent heat from the freezing of ground moisture is accounted for heat in  $c_p^{\text{eff}}$  in all scenarios, so this quantity is never fully constant.

The resulting radius and evaporation rate can be seen in Figs. 8a and 8b, where we observe similar time until complete boil-off, but smaller radius for variable thermal properties. However, the difference in maximal radius is less than 12 %. This result can be explained by the opposing effects of increasing thermal conductivity and decreasing specific heat capacity as a function of decreasing temperature. Therefore, constant thermal properties may be sufficient, provided their values are evaluated at an appropriate temperature. There may, however, be substrates with properties that do not facilitate such a procedure.

# 3.2.2. Cases A and C: Perfect thermal contact vs. boiling correlations

A common simplification practice in the literature is the assumption of perfect thermal contact between the ground and the liquid (Holborn *et al.*, 2020; Verfondern and Dienhart, 1997). This gives the maximal possible heat flux between the liquid and the ground. However, in reality, there will be a finite temperature difference between the surface and the evaporating liquid, where the heat flux is typically described by a boiling correlation for the relevant boiling regime. For a large temperature difference, typically the case for LH<sub>2</sub>, the heat transfer happens through film boiling, having an insulating vapour film between the substrate and the liquid. On the other hand, for LNH<sub>3</sub>, the heat transfer is typically within the nucleate boiling regime in which bubbles form at and separate from the ground surface. To account for the different boiling regimes that LH<sub>2</sub> and LNH<sub>3</sub> experience, we employ the boiling correlations in Eqs. (20) and (26), respectively.

The difference between assuming perfect thermal contact and accounting for the boiling regimes is displayed in Figs. 9a and 9b where spill Cases A and C for  $LH_2$  are compared. Perfect thermal contact yielded a higher initial evaporation rate, and the spill evaporated faster. However, with time, the evaporation rates of the two cases approach each other. This shows that the ground's thermal conductivity is a primary factor limiting evaporation once the top layer of the ground has cooled. Hence, the simulations of short-duration spills are more affected by the simplifying assumption of perfect thermal contact.

As seen in Figs. 9c and 9d, the predicted behaviour was similar for LNH<sub>3</sub>, albeit with a considerably longer time to complete boil-off, and hence, a larger absolute difference in boil-off time between the cases of perfect thermal contact and nucleate boiling. Despite the marginal differences in radius and evaporation rate, we observed a notable difference in the temperature profiles in the ground. This implies that large differences in temperature profiles do not necessarily give rise to large differences in



Figure 7: Release cases A and B with LH<sub>2</sub>. (a) Pool radius. (b) Evaporation rate (full lines) on left second axis and total evaporated mass (dashed lines) on right second axis.

radius and evaporation rate.

#### 3.2.3. Cases A, D, E and F: Dry vs. wet substrate

In the following, we study the effect of ground moisture on radius, evaporation rate and temperature profiles in the ground. Spill Cases A and D are compared in Fig. 10a which shows that for  $LH_2$  the pool radius will be smaller for wet saturated sand compared to dry sand. This is reflected in a higher evaporation rate for wet sand in Fig. 10b.

As expected, we observe the same behaviour for  $LNH_3$  in Figs. 10c and 10d, but with a significantly more pronounced difference in radius, evaporation rate and time before complete boil-off. The time before complete boil-off was one third for wet sand compared to dry sand.

The large differences between wet and dry sand for LNH<sub>3</sub> seen in Figs. 10c and 10d are mainly explained by the release of heat associated with freezing of ground moisture. This in turn maintains a larger temperature difference  $\Delta T_{g\ell}$ , which in turn causes a higher heat flux from the ground. Additionally, the increase in thermal conductivity  $\lambda_g$  for wet saturated sand may enhance the evaporation compared to dry sand. For LNH<sub>3</sub>, the ground was maximally cooled to  $-27 \,^{\circ}$ C for wet saturated sand, while for LH<sub>2</sub> it was maximally cooled to  $-98 \,^{\circ}$ C. Hence, for LH<sub>2</sub>, the relative contribution from cooling is higher, and thus the latent heat is less significant.

 $LNH_3$  on concrete: Concrete is often used as substrate, both for storage of  $LNH_3$  and  $LH_2$ , and in experiments. Thermal properties of concrete are plotted in Figs. 2a and 2b, but data are only at temperatures relevant to  $LNH_3$  spills. The resulting radius and evaporation rate for spill Cases E and F are given in Figs. 11a and 11b. Compared to dry concrete, the time until complete boil-off is one third shorter for concrete that was first dried and then wetted (i.e., rewetted). This illustrates again that evaporation rates for dry and wet substrates are significantly different, and must be accounted for



Figure 8: Continuous axisymmetric release of LH<sub>2</sub> on wet sand over 1200 s with spill rate  $\dot{m}_{\ell} = 9.5 \text{ kg/s}$  inside a radius of 0.75 m. (a) Pool radius. (b) Evaporation rate (full lines) on left second axis and total evaporated mass (dashed lines) on right second axis. Plotted for constant properties at  $T = -1 \degree \text{C}$  (blue) and variable thermal properties (orange). Freezing of ground moisture is accounted for in both cases.

when performing hazard and operability studies.

#### 3.2.4. Case X: Complex case

Finally, as a demonstration of the tool, showing its versatility, we simulate a continuous timedependent spill of LNH<sub>3</sub> with mass spill rate (kg/s)  $\dot{m}_{\ell} = 15 - 0.25t$  inside a rectangular domain with an initial velocity  $u_0 = 1$  m/s, see Fig. 12. This is similar to the release from a non-pressurized storage tank. The spill is constrained by a wall for  $x \le -2$  m. Two obstacles are included, serving as pillars holding up a roof between the wall and the pillars. The substrate is dry concrete below the roof and wet saturated sand outside the roof. The height and local evaporation rate at t = 10 s are shown in Figs. 13a and 13b. Both variable ground properties and the boiling correlation in Eq. (26) have been employed. Here we observe how wet saturated sand causes a higher local evaporation rate compared to dry concrete.

This demonstration case shows that the model can be applied to complex geometries relevant to real storage and transport infrastructure. Such cases could prove important both for detailed analysis of the development and extent of the cold spill, and as input to improved analysis of the formation and spread of the hazardous cloud.



Figure 9: Release Cases A and C for  $LH_2$  (left column) and  $LNH_3$  (right column). (a) and (c) Pool radius. (b) and (d) Evaporation rate (full lines) on left second axis and total evaporated mass (dashed lines) on right second axis.

#### 4. Conclusions

In this work, a new, flexible tool has been developed to simulate the spreading and continuous evaporation of cryogenic liquids, such as LH<sub>2</sub> and LNH<sub>3</sub>, on land. The tool can account for various spill and substrate details, including transient spill rates, the initial momentum of the spill, the topography of the ground, obstructions, and temperature-dependent thermal properties of the substrate.

Exploiting the fact that the heat conduction is mainly vertical and initiated by the spill's wetting, we have implemented the substrate heat conduction as a pre-processing step, reducing computational times significantly.

The tool's validation against experiments highlighted the importance of understanding the thermal properties of the ground. Moreover, the constant-thermal-properties assumption was found reasonable for both  $LH_2$  and  $LNH_3$  spills, provided the substrate thermal properties are evaluated at frozen-soil temperatures and that the latent heat of substrate moisture is accounted for. Similarly, assuming perfect thermal contact or using a boiling correlation did not give significant differences in the spill-evaporation development. Both these assumptions should be checked if other substrates are considered, or for long-duration spills.

Furthermore, the effect of ground moisture was investigated. The water content in the substrate was found to have a significant impact on the spill spread and evaporation. The effect was found to be particularly pronounced for LNH<sub>3</sub>, with a relative increase in peak evaporation rate of 1.5 or 3 when concrete or sand was wetted, respectively. This highlights the importance of accounting for ground moisture when predicting the size and evaporation rate of cryogenic pools. These findings can inform more accurate hazard and operability studies for cryogenic storage and handling facilities.



Figure 10: Release Cases A and D for  $LH_2$  (left column) and release Cases A and D for  $LNH_3$  (right column). (a) and (c) Pool radius. (b) and (d) Evaporation rate (full lines) on left second axis and total evaporated mass (dashed lines) on right second axis.

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Figure 11: Release Cases E and F for LNH<sub>3</sub>. (a) Pool radius. (b) Evaporation rate (full lines) on left second axis and total evaporated mass (dashed lines) on right second axis.



Figure 12: Sketch of complex Case X where  $LNH_3$  is released on dry concrete (grey) under a roof (blue) and subsequently reaches wet saturated sand (brown). A wall (blue) and pillars (black) constrain the spill.



Figure 13: Top-down view of release Case X (Fig. 12). Time-dependent continuous spill of LNH<sub>3</sub> at a rate (kg/s)  $\dot{m}_{\ell} = 15 - 0.25t$  inside a rectangular domain of size  $x \times y = 1 \text{ m} \times 0.75 \text{ m}$  with initial velocity  $u_0 = 1 \text{ m/s}$ . Obstacles are displayed in hatched grey. The substrate is dry concrete ( $x \le 2.1 \text{ m}$ ) and wet saturated sand ( $x \ge 2.1 \text{ m}$ ). (a) Height of spill at t = 10 s. (b) Local evaporation rate at t = 10 s.

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